AUBURN STORMWATER **AU-STORMWATER RESEARCH FACILIT** UPDATE AND **INFILTRATION SWALE** RESEARCH Wesley N. Donald, PhD

> September 25-26, 2024 Landmark Park Dothan, Alabama





The Law: Clean Water Act - 1972

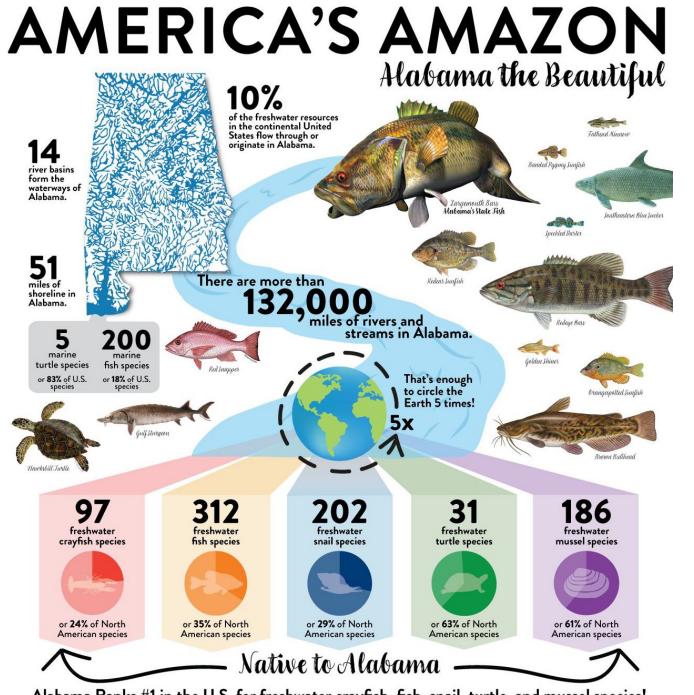
Purpose: Restore and maintain chemical, physical, and biological integrity of our Nation's waters.

Prohibition of toxic pollutants

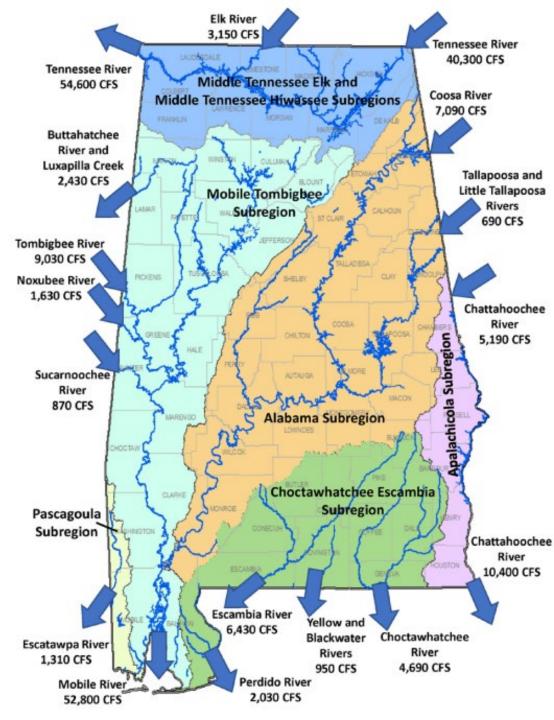
"Fishable" and "swimmable" goal by 1983

Elimination of pollutant discharge by 1985

Modified in 1987 to create NPDES & MS4



Alabama Ranks #1 in the U.S. for freshwater crayfish, fish, snail, turtle, and mussel species!





BEACH BASICS BEACH FINDER ABOUT V



Orange Beach Waterfront Park, Wolf Bay Met water quality standards less than 60% of the time More info



Current Water Quality 🚯

Failed to meet water quality standards

This status is based on the latest sample, take on July 5th, 2024 Mobile Baykeeper updates the status of this beach as soon as test results become available. These results were posted to Swim Guide on July 5th at 1:19 PM.

Monitoring Frequency:

G

Orange Beach Waterfront Park, Wolf Bay is sampled Weekly from March 10th to October 1st

Passed Water Quality Tests



Chiques Legal

 2012
 Passed Water Quality Tests: 88.9%

 80%
 Passed Water Quality Tests: 88.9%

 60%
 Passed Water Quality Tests: 88.9%

 50%
 Passed Water Quality Tests: 88.9%

 60%
 Passed Water Quality Tests: 88.9%

 2012
 2013

 2012
 2013

 2012
 2013

 2012
 2013

 2014
 2015

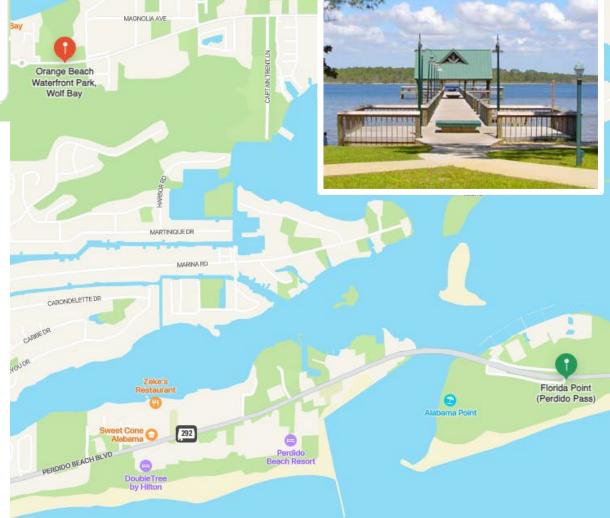
 2016
 2017

 2018
 2019

 2020
 2021

 2020
 2021

 2020
 2021





EVALUATING INFILTRATION SWALE PERFORMANCE

Alabama Department of Transportation

BACKGROUND

POST CONSTRUCTION INFILTRATION SWALES

1. Vegetated Open Channel

2. Engineered Soil Media Matrix

4. Check Dam

Engineered system that promotes groundwater infiltration and reduces surface runoff

3. Native Surrounding Soil

INFILTRATION SWALES

Engineered Soil Media Matrix Reduces runoff Mimics pre-hydrology

Promotes infiltration

SMALL-SCALE TESTING



SMALL-SCALE TESTING



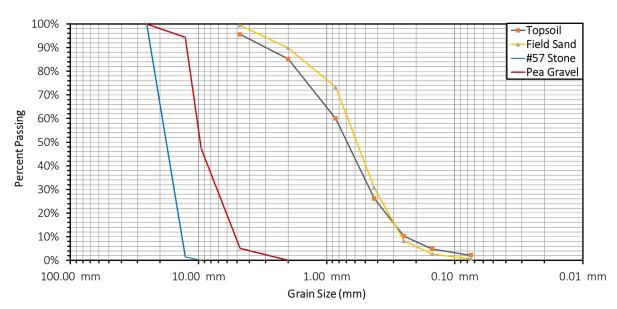


- Material properties
 - Gradation
 - Porosity
 - Bulk unit weights
 - Compaction
- Permeability testing
- Infiltration testing
 - Falling head test
 - Constant head test

TESTING MATERIALS



GRADATION, POROSITY, & BULK DENSITY



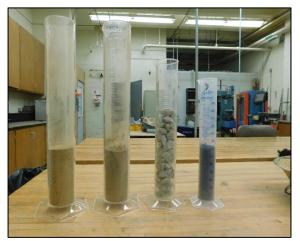








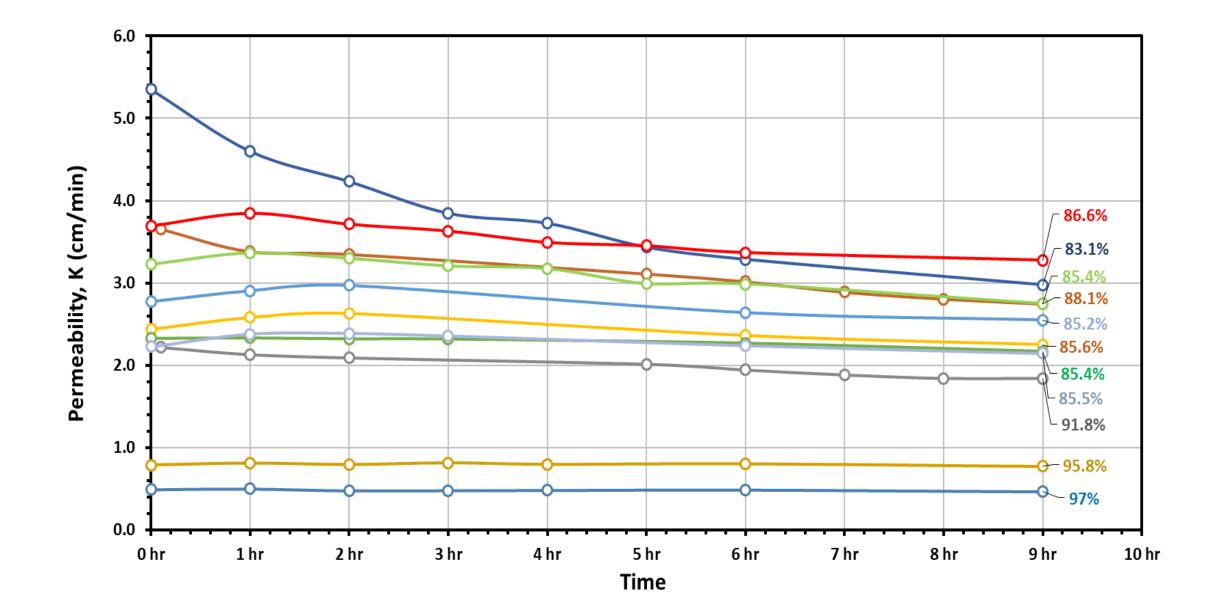
	POROSITY:	LOOSE SAMPLES				
Sample	Graduated cylinder Weight (g)	G. Cylinder + Dry sample Weight (g)	Dry Sample weight (g)	Sample volume (cm3)	Water Volume Added (cm3)	Porosity (%)
Field sand	217.4	731.1	513.7	330	127	38.5%
Top Soil	216.8	659.4	442.6	315	128.9	40.9%
#57 Stone	157.7	601.5	443.8	450	235.8	52.4%
Ever green Top soil	86.8	109.5	22.7	130	95.6	73.5%



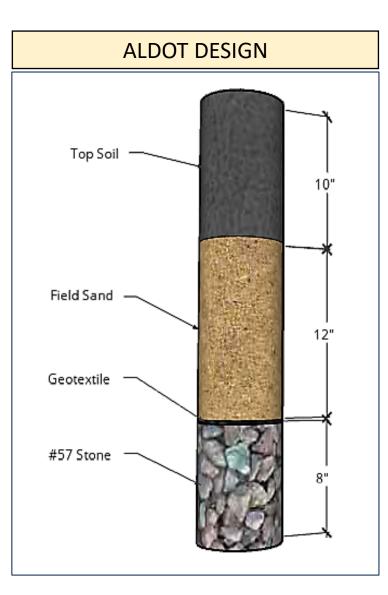


	BULK DENSITY:	LOOSE SAMPLES			
Sample	Graduated cylinder Weight (g)	G. Cylinder + Dry sample Weight (g)	Dry Sample weight (g)	Sample volume (cm3)	Bulk density (g/cm3)
Field sand	217.4	716.7	499.3	350	1.43
Top Soil	216.8	644.8	428	330	1.30
#57 Stone	157.6	651	493.4	405	1.22
Ever green Top soil	86.8	110	23.2	135	0.17

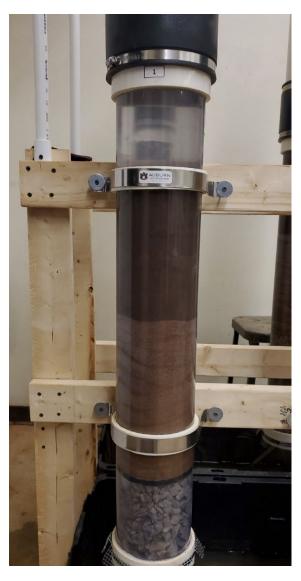
PERMEABILITY OF FIELD SAND VS TIME AT DIFFERENT DENSITIES



ALDOT INFILTRATION SWALE SAMPLES



<u>Column 1</u>



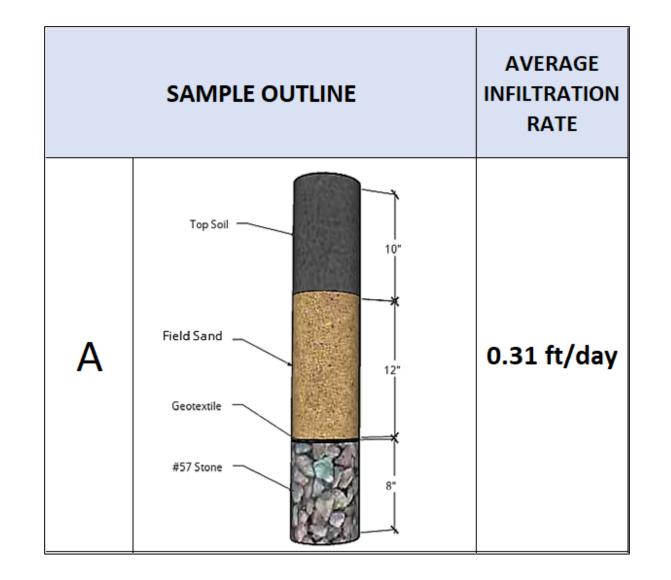
Column 2



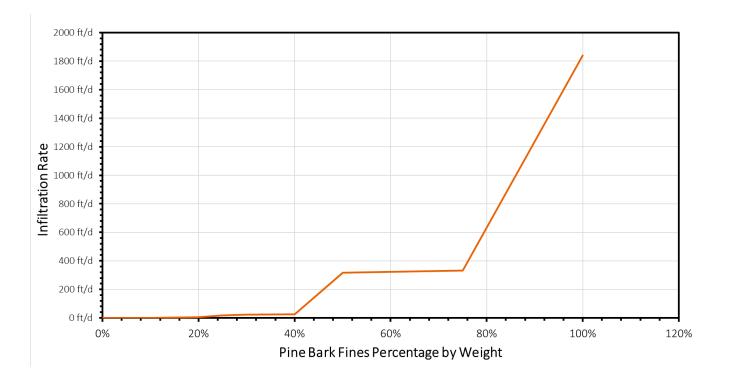
Column 3



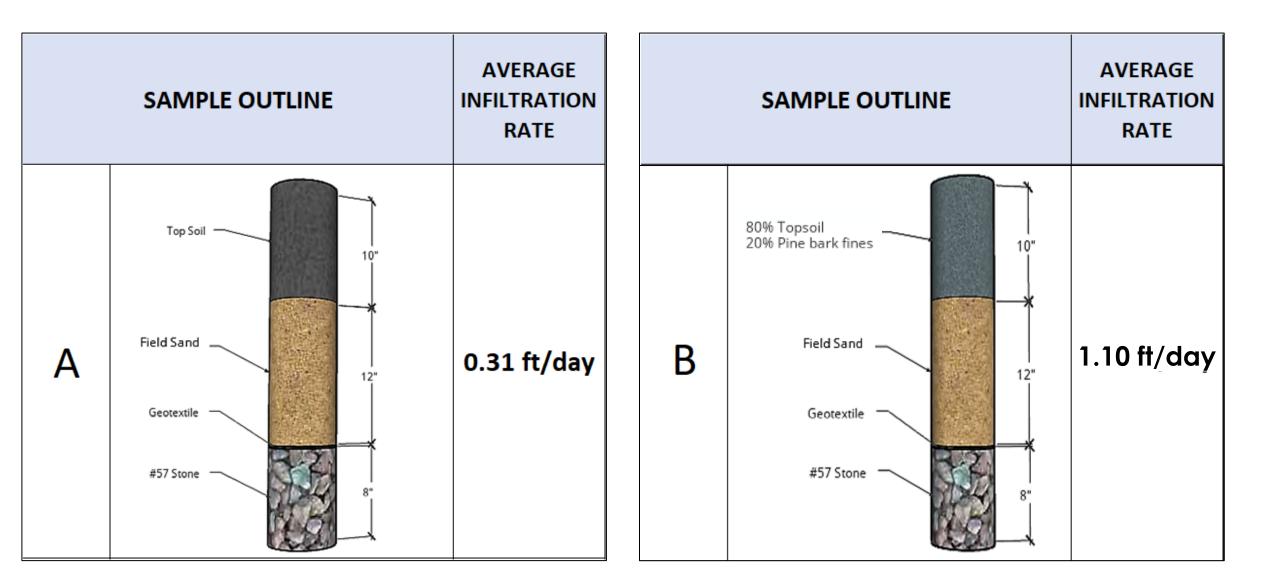
SAMPLES TESTED – FALLING HEAD TEST



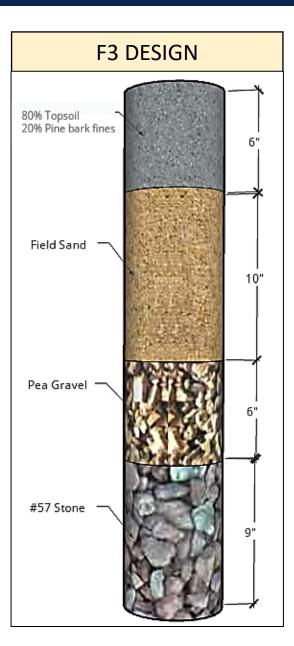
Pine Bark Fines	Top Soil	Infiltration rate
0%	100%	0.63 ft/day
5.0%	95%	0.76 ft/day
7.4%	93%	0.89 ft/day
10%	90%	1.14 ft/day
15%	85%	2.37 ft/day
20%	80%	5.60 ft/day
25%	75%	17.54 ft/day
30%	70%	26.23 ft/day
40%	60%	25.61 ft/day
50%	50%	317.66 ft/day
75%	25%	331.08 ft/day
100%	0%	1840.00 ft/day



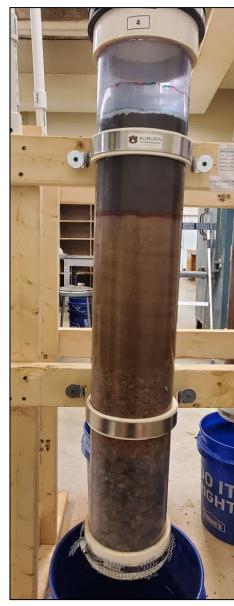
SAMPLES TESTED – FALLING HEAD TEST



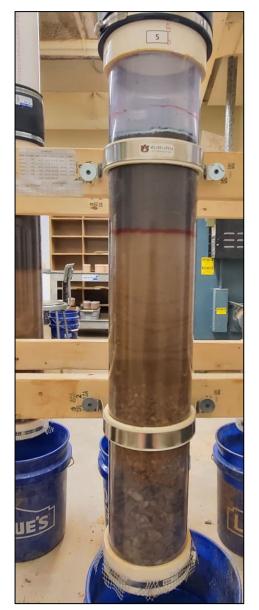
F3 SAMPLE POTENTIAL NEW DESIGN



Column 1



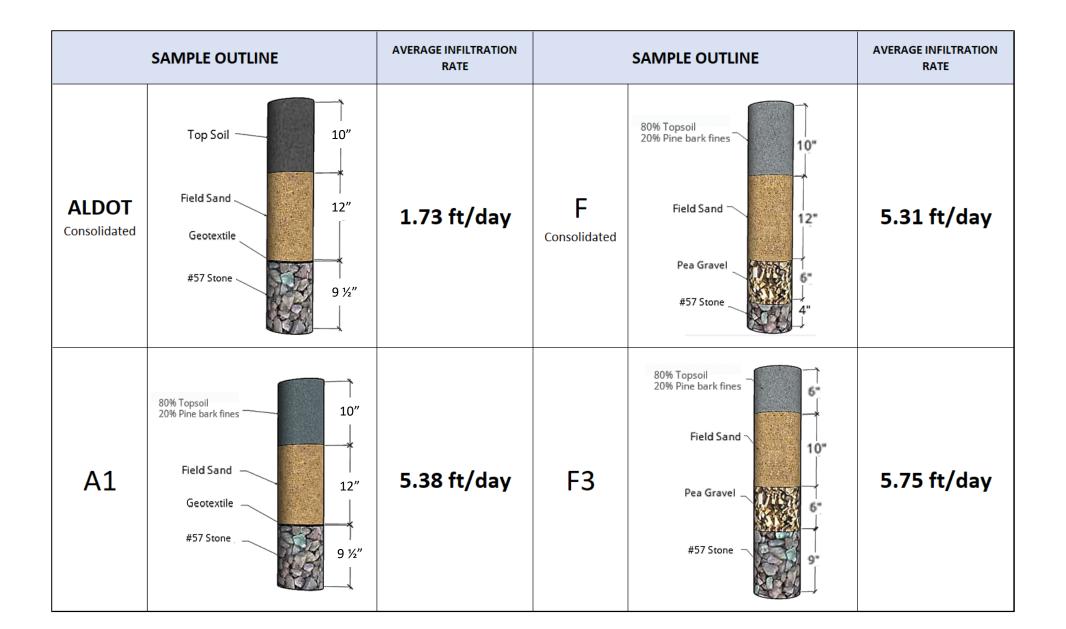
<u>Column 2</u>



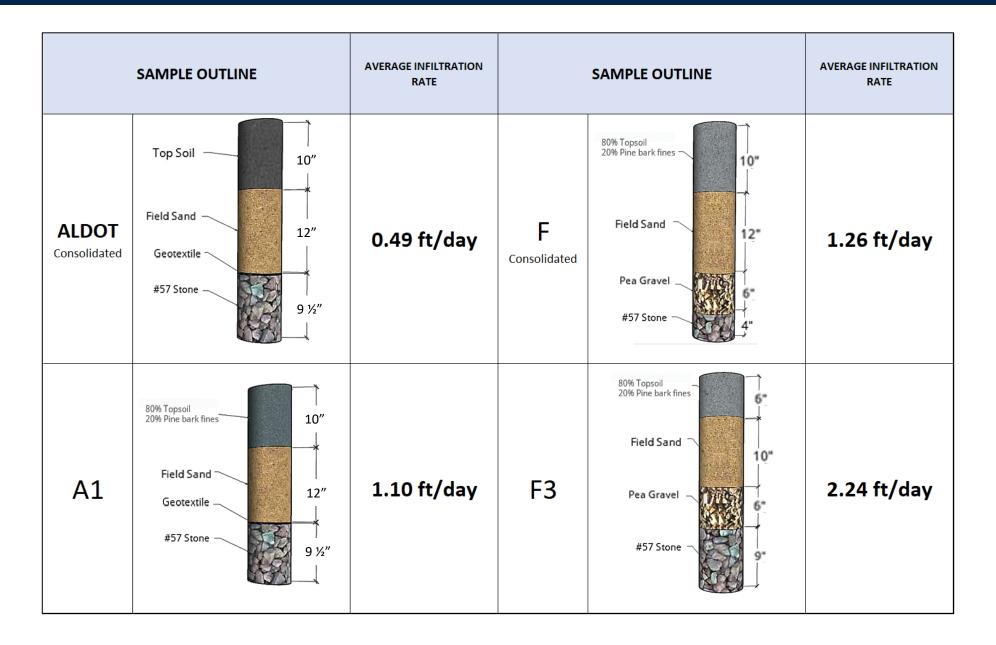
<u>Column 3</u>



BEST SAMPLES - CONSTANT HEAD TEST

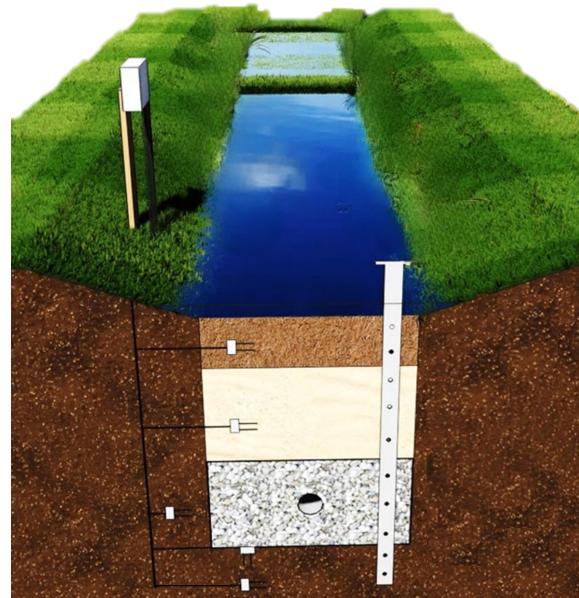


BEST SAMPLES – FALLING HEAD TEST

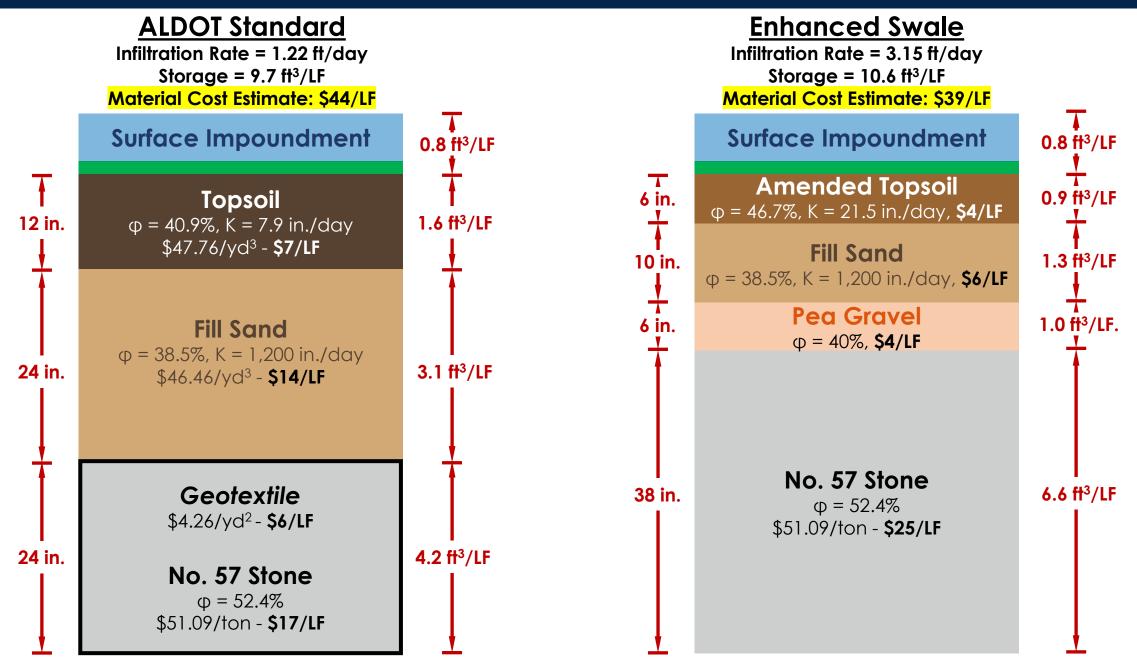


COLUMN TESTING FINDINGS > FIELD APPLICATION

- Topsoil is limiting layer
- 80/20 pine bark fines amendment improves infiltration
 - Increased permeability by 9x
 - Column test infiltration improved by 2.6 to 3.1x
- Consider reducing 12 in. topsoil layer to 6 in.
- Geotextile reduces infiltration rate
 - Pea gravel increased infiltration rate by 2.2 to 3.1x



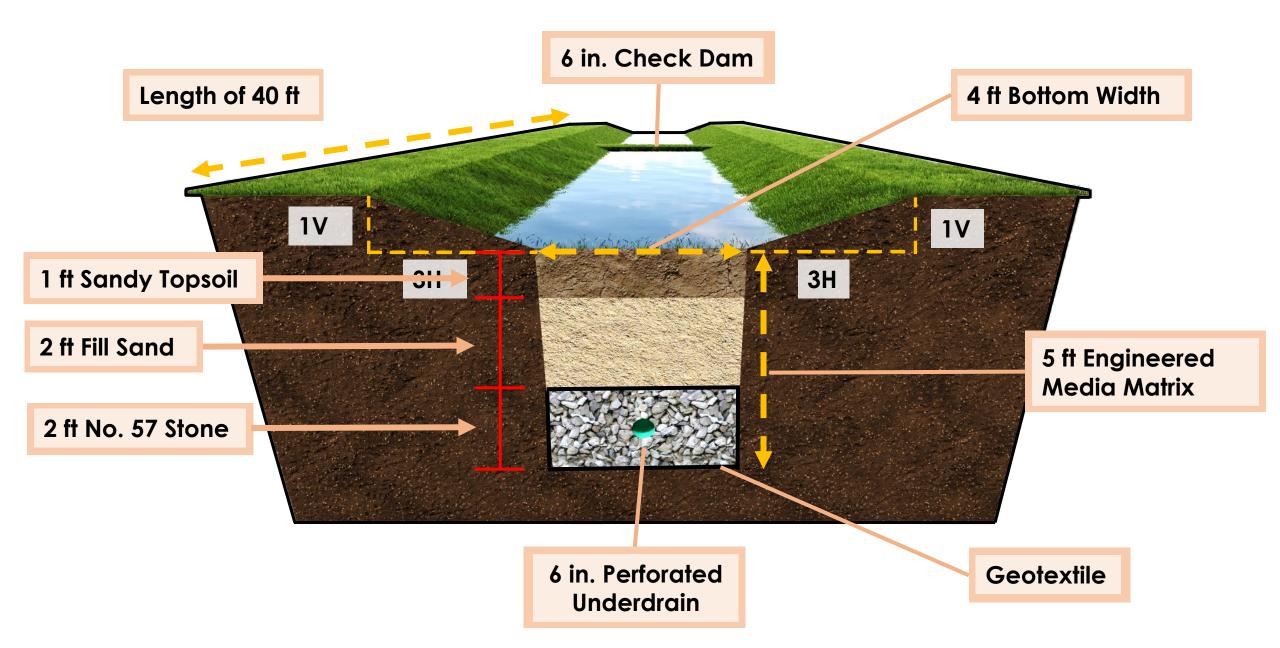
COST COMPARISON



FIELD SCALE TESTING PHASE



ALDOT INFILTRATION SWALE DESIGN



ALDOT SWALE SITE SELECTION

Double Ring Infiltrometer

Hydrologic Soil Group	Infiltration Rate (in/hr)	Infiltration Rate (cm/hr)	Soil Textures
A	1.63	4.14	Silty Gravels Gravelly Sands Sand
A	0.8	2.03	Sand Loamy Sand Sandy Loam
	0.45	1.14	Silt Sands
В	0.3	0.76	Loam Silt Loam
С	0.2	0.51	Sandy Clay Loam Silts
D	0.06	0.15	Clay Loam Silty Clay Loam Sandy Clay Silt Clay Clay

Table: MnDOT



DOUBLE RING INFILTROMETER RESULTS

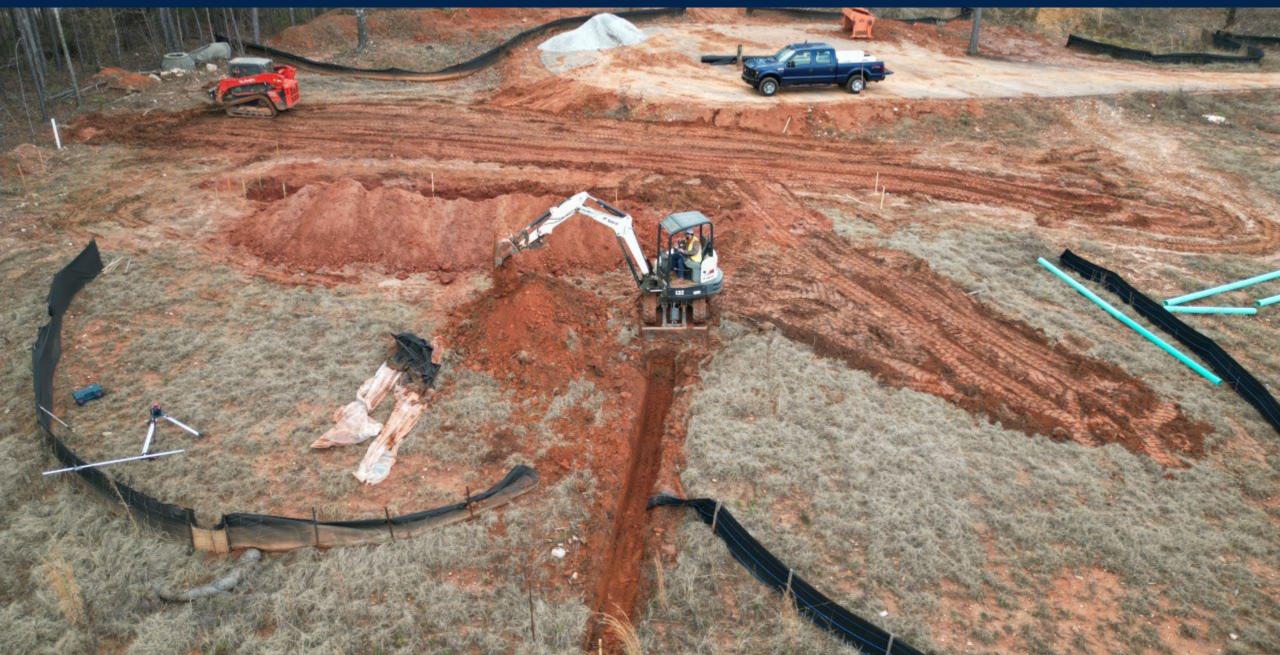
_	_	Hydrologic Soil Group	Infiltration Rate (in/hr)	Infiltration Rate (cm/hr)	Soil Textures
Double Ring V Time (min) Initial (cn			1.63	4.14	Silty Gravels Gravelly Sands Sand
0-30	90.0	A			Sand
<u> </u>	90.0 90.0		0.8	2.03	Loamy Sand Sandy Loam
90-120	91.0		0.45	1.14	Silt Sands
120-150 150-180	90.0 90.9	В	0.3	0.76	Loam Silt Loam
180-110	91.9 e Infiltratio	С	0.2	0.51	Sandy Clay Loam Silts
Safety Fc		D	0.06	0.15	Clay Loam Silty Clay Loam Sandy Clay Silt Clay Clay

Infiltration Soil Classification: Native Soil – Sandy Loam Composition HSG B

SWALE CONSTRUCTION



EXCAVATION



CHANNEL CONSTRUCTION



CONSTRUCTION OF ALDOT INFILTRATION SWALE



5 ft Excavation

Geotextile

2 ft #57 and Underdrain

Closed #57 Stone

CONSTRUCTION OF ALDOT INFILTRATION SWALE



2 ft Sand

1 ft Topsoil

1% Grade & Channel Shaping

STABILIZED SWALE



LARGE WEIR BOX CONSTRUCTION AND INSTALLATION



SWALE VOLUMES WERE VERFIED



TEROS 10 MOISTURE CONTENT SENSORS

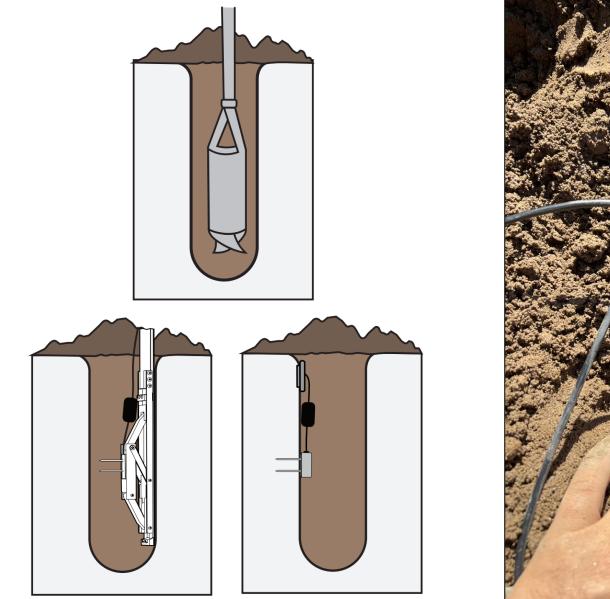
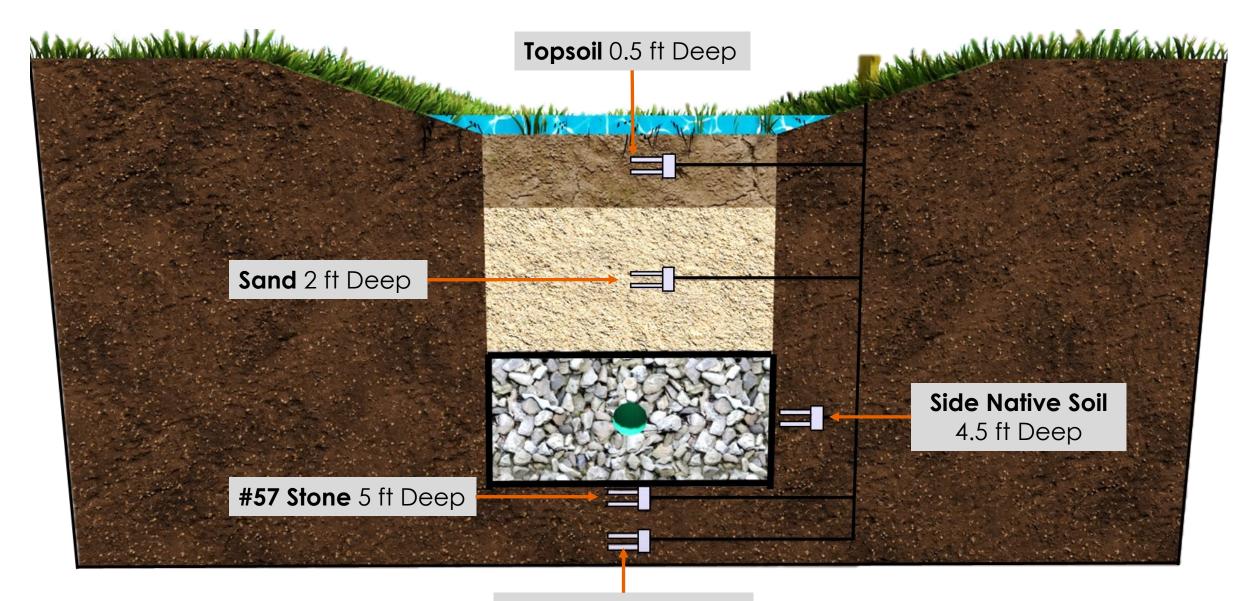


Figure: METER Group

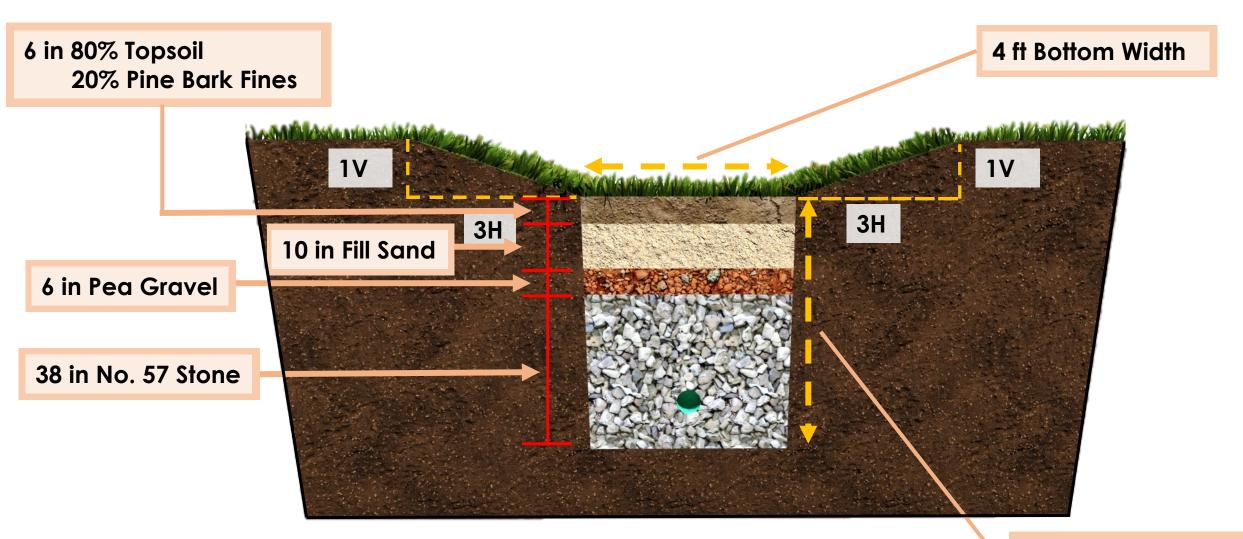


ALDOT MOISTURE CONTENT SENSORS



Native Soil 8 ft Deep

MODIFIED SWALE INFILTRATION SWALE DESIGN

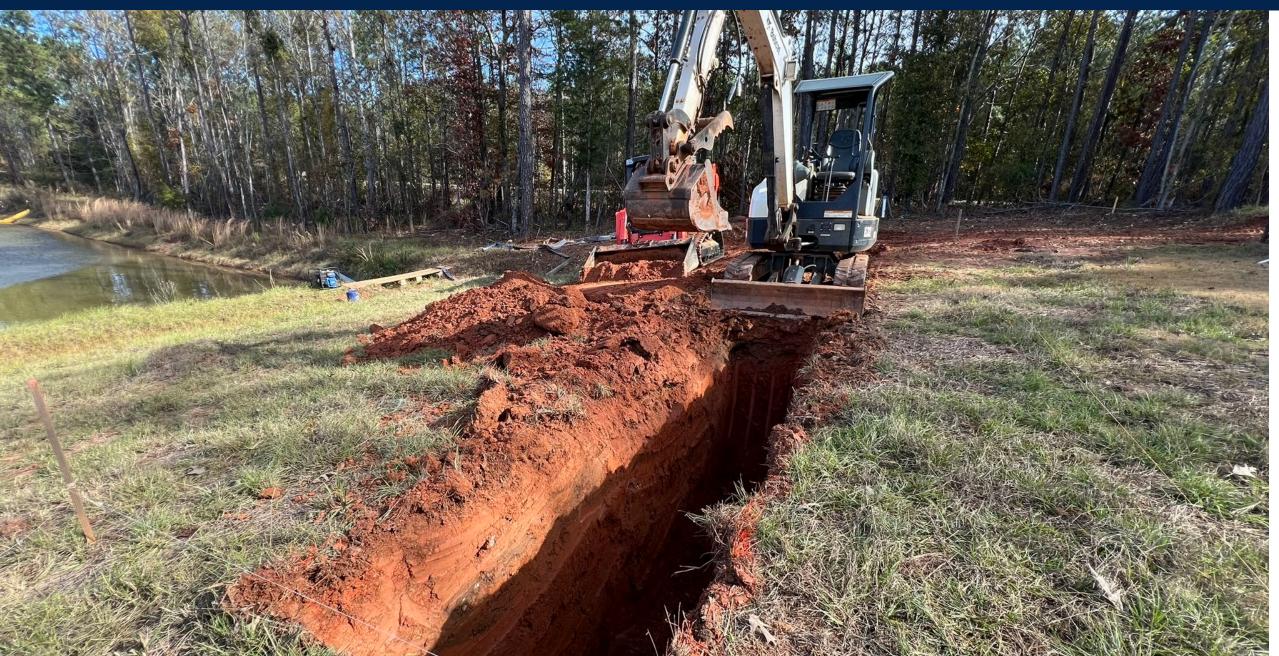


5 ft Engineered Media Matrix

MODIFIED SWALE SITE LAYOUT



MODIFIED SWALE EXCAVATION



UNDERDRAIN AND NO.57 STONE



UNDERDRAIN AND NO.57 STONE



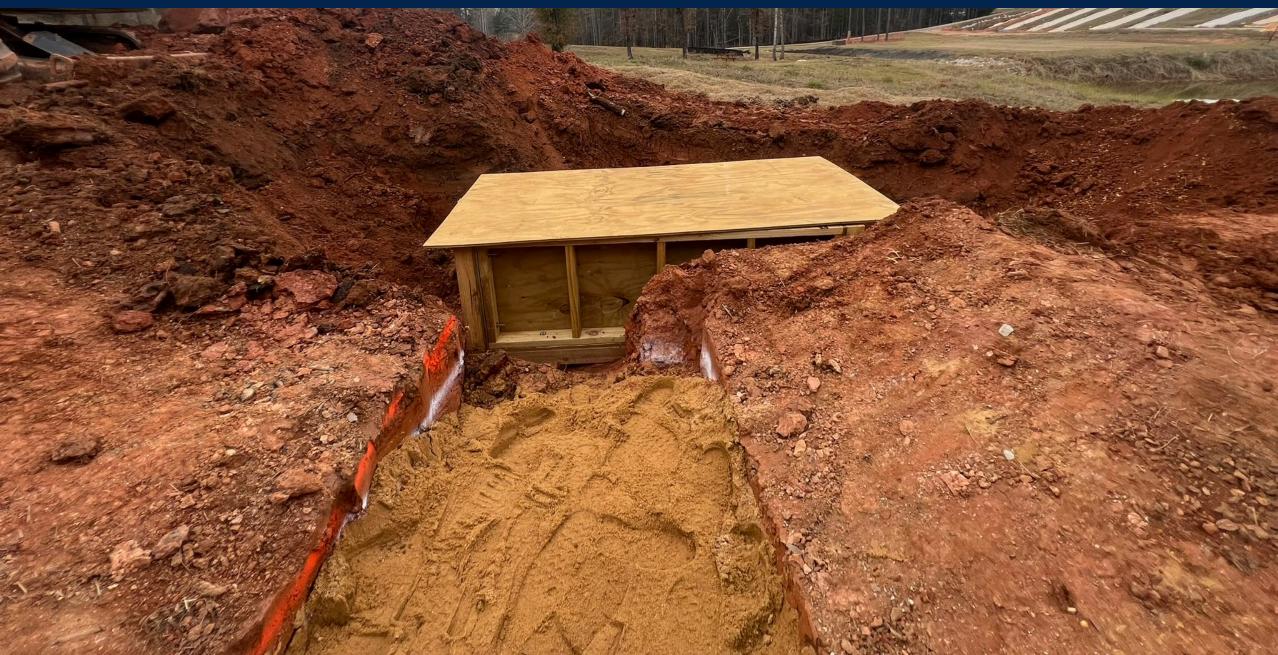
PEA GRAVEL LAYER



FILL SAND LAYER



SURFACE WEIR BOX



TOPSOIL LAYER



FINAL GRADING AND SWALE SHAPING



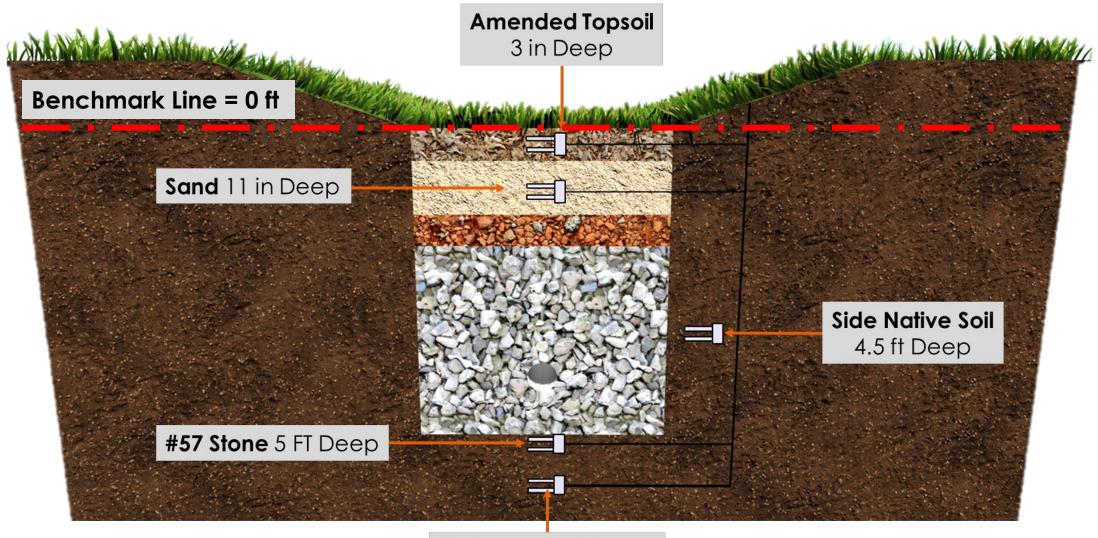
BERMUDA TIFWAY SODDING



READY FOR TESTING



SWALE MOISTURE CONTENT SENSORS



Native Soil 8 ft Deep

RESEARCH TESTING

Experimentation to Understand Factors that Affect Performance:

1. Infiltration swale drawdown times

2. Moisture content sensor data

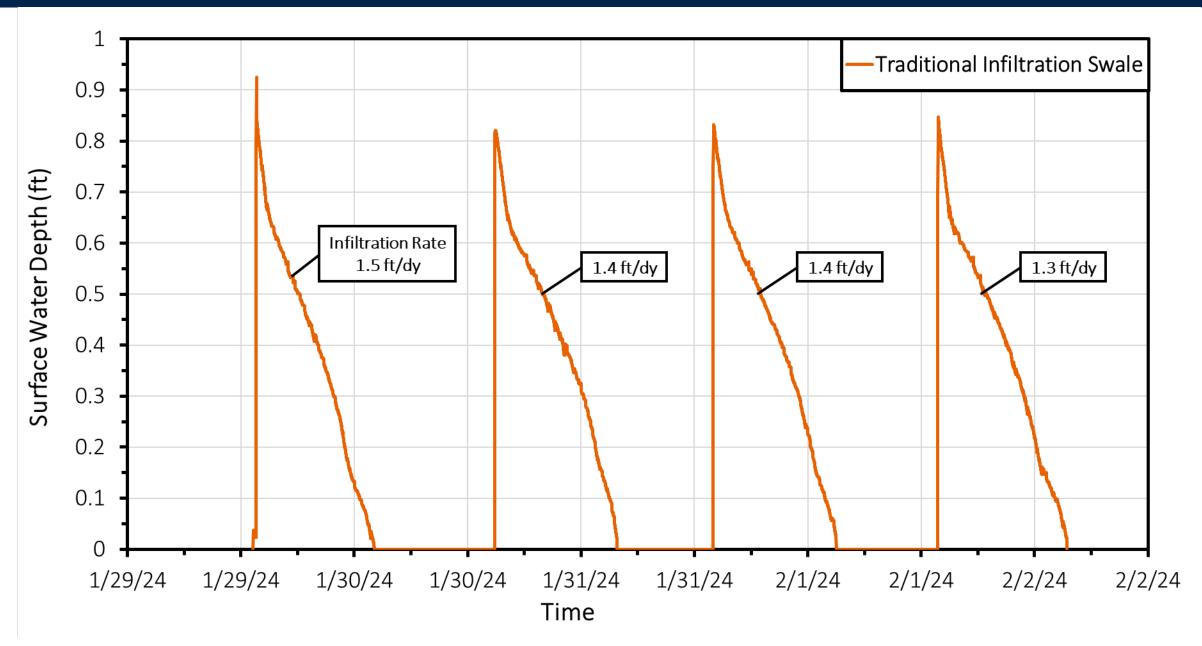
3. Infiltration rates

4. Settlement

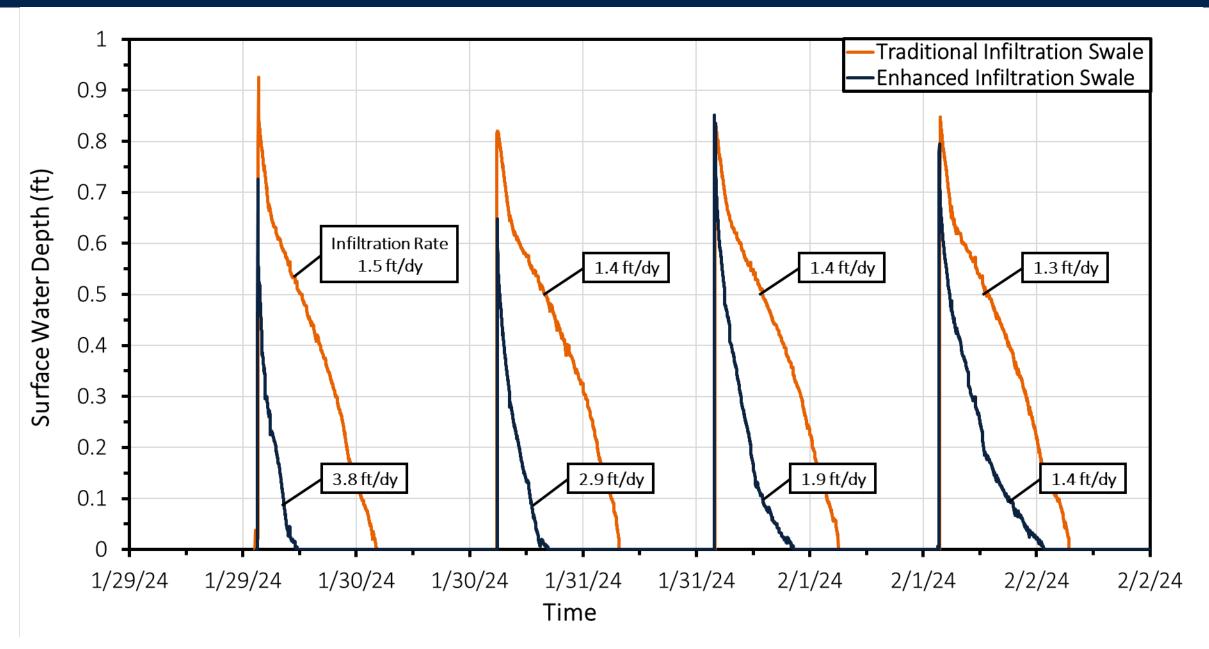
1. INFILTRATION SWALE DRAWDOWN



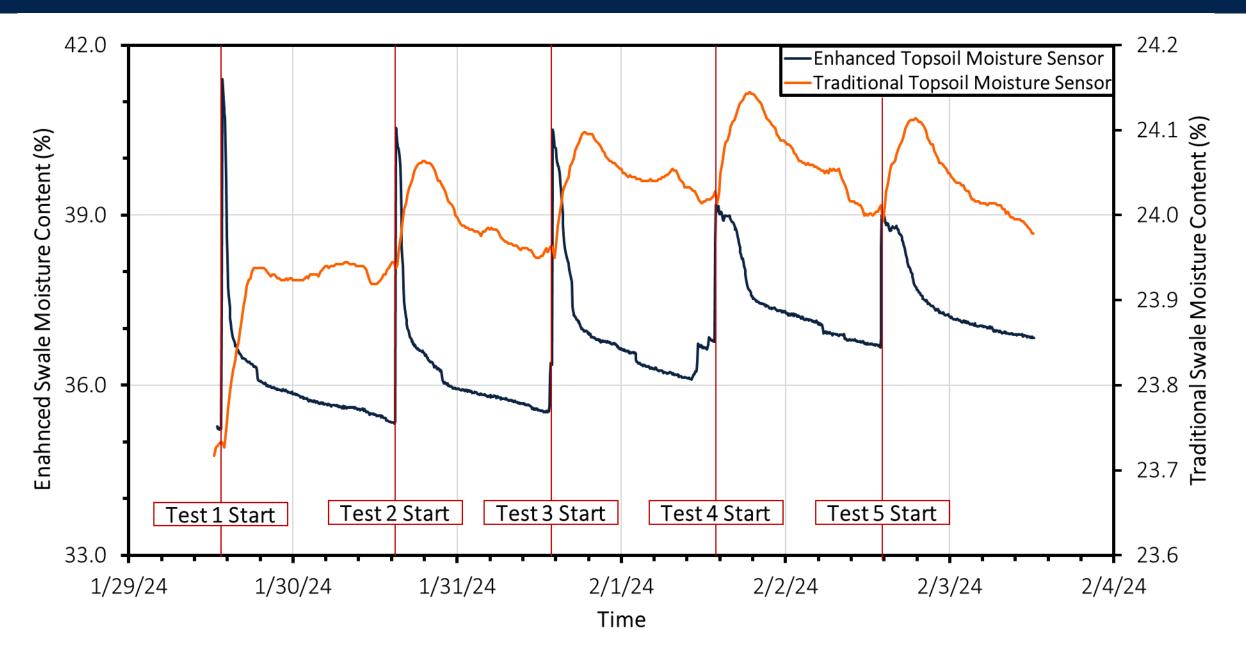
ALDOT SWALE DRAWDOWN



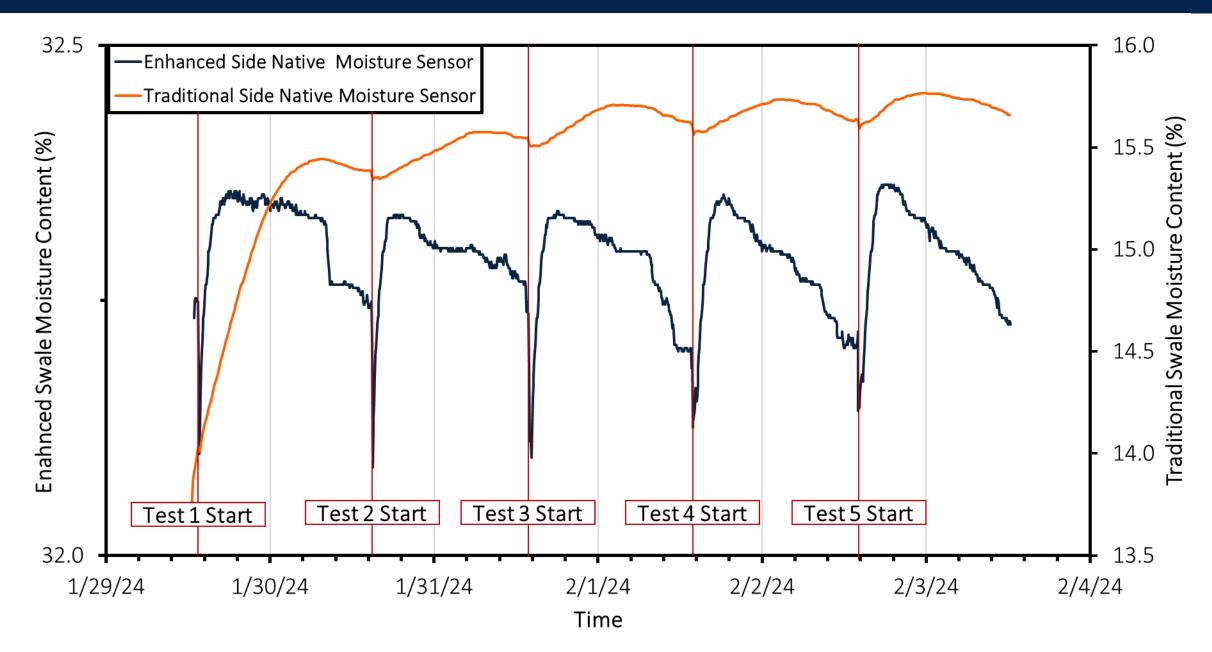
DRAWDOWN COMPARISON



TOPSOIL MOISTURE CONTENT



SIDE NATIVE MOISTURE CONTENT



FIELD SCALE MAJOR FINDING

ALDOT Infiltration Swale





Infiltration Rate: 1.22 ft/d Drawdown Time: 16.1 hr



Infiltration Rate: 3.15 ft/d

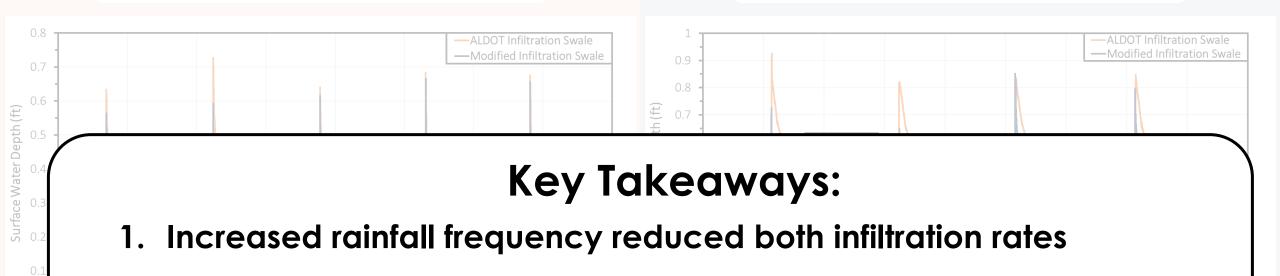
Drawdown Time: 6.6 hr

Enhanced Swale draws down 2.5x faster than ALDOT Swale

INFILTRATION EVALUATION – ONE DAY VS. THREE DAY

3 DAY DRY PERIOD

1 DAY DRY PERIOD



- 2. Modified swale outperformed the ALDOT swale in both frequencies
- 3. The Modified swale saw a larger reduction in infiltration rates
- Modified avg. infiltration rate: **5.88 ft/day**
- Modified swale **2.6x** faster

- Modified avg. infiltration rate: 2.5 ft/day
- Modified swale 1.8x faster

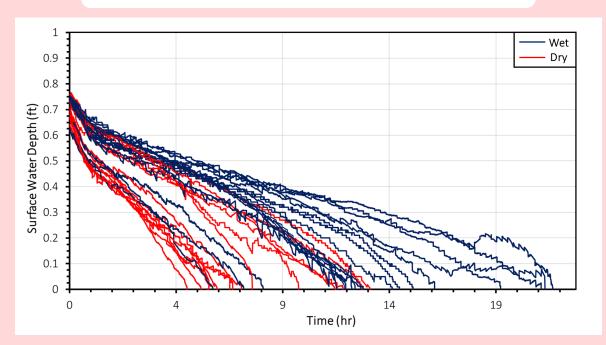
WET VS. DRIER SOILS

Δ5

hrs

ALDOT SWALE

MODIFIED SWALE

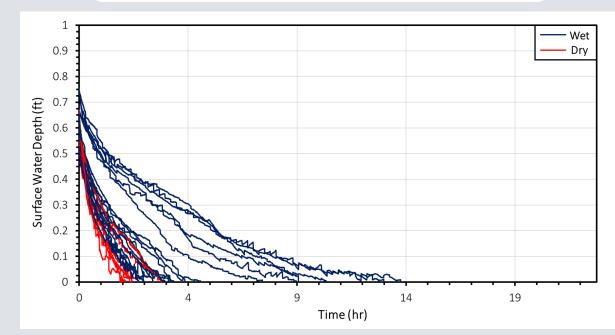


Wet Soil Conditions:

- Average infiltration rate: 1.4 ft/day
- Average drawdown time: 13.7 hours

Drier Soil Conditions:

- Average infiltration rate: 2.1 ft/day
- Average drawdown time: 8.7 hours



Wet Soil Conditions:

- . Average infiltration rate: 2.5 ft/day
- Average drawdown time: **8.1 hours**

Drier Soil Conditions:

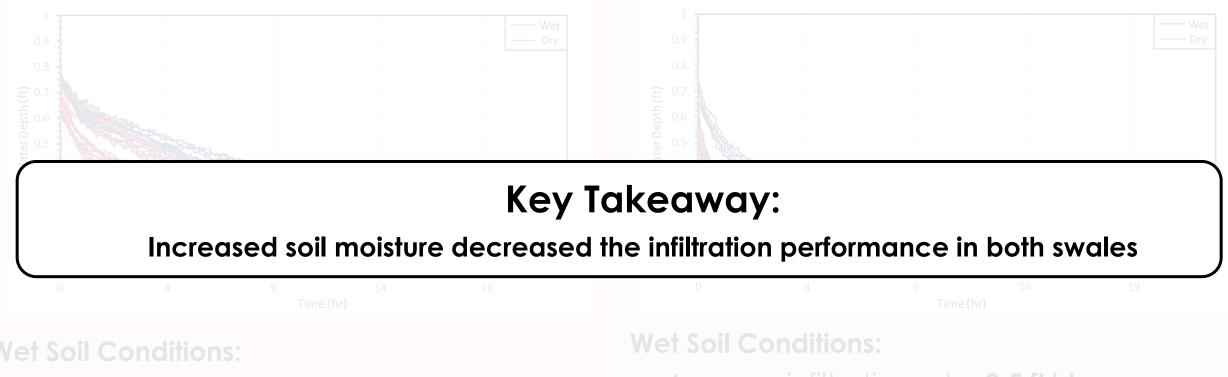
- Average infiltration rate: 5.8 ft/day
- Average drawdown time: 2.7 hours

∆5.4 hrs

WET VS. DRIER SOILS

ALDOT SWALE

MODIFIED SWALE

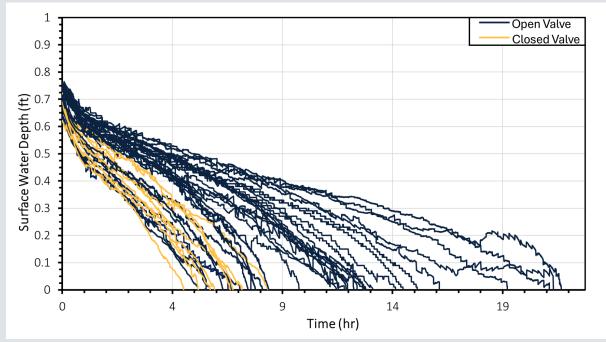


- Average infiltration rate: 1.4 ft/day
- Average drawdown time: 13.7 hours
 Drier Soil Conditions:
- Average infiltration rate: 2.1 ft/day
- Average drawdown time: 8.7 hours

- Average infiltration rate: 2.5 ft/day
- Average drawdown time: 8.1 hours
- Drier Soil Conditions:
- Average infiltration rate: 5.8 ft/day
- Average drawdown time: 2.7 hours

ALDOT SWALE

MODIFIED SWALE

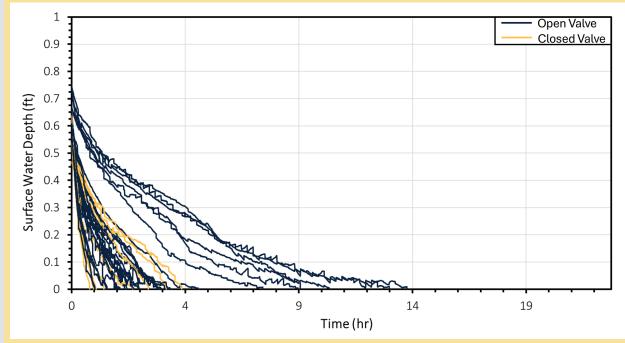


Open Valve:

- Average infiltration rate: 1.6 ft/day
- Average drawdown time: 12 hours

Closed Valve:

- Average infiltration rate: 2.5 ft/day
- Average drawdown time: **7 hours**



Open Valve:

- Average infiltration rate: **5.2 ft/day**
- Average drawdown time: **5 hours**
- **Closed Valve:**

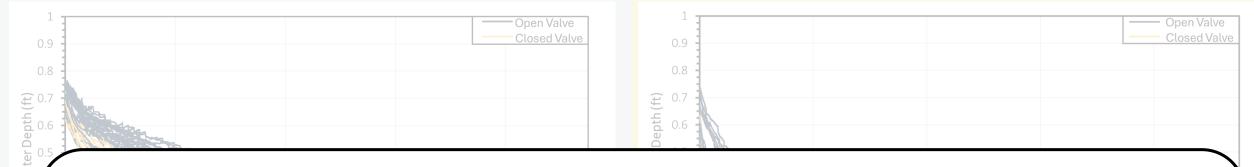
Δ5

hrs

- Average infiltration rate: 9.5 ft/day
- Average drawdown time: 2.3 hours (
- ∆2.7 hrs

ALDOT SWALE

MODIFIED SWALE



Key Takeaways:

- 1. Closed valve tests outperformed open valve contrary to prediction
- 2. Closed valve tests were performed in warmer months
- 3. Leads to investigate if seasonal variation is the cause for results

 $\Delta 5$

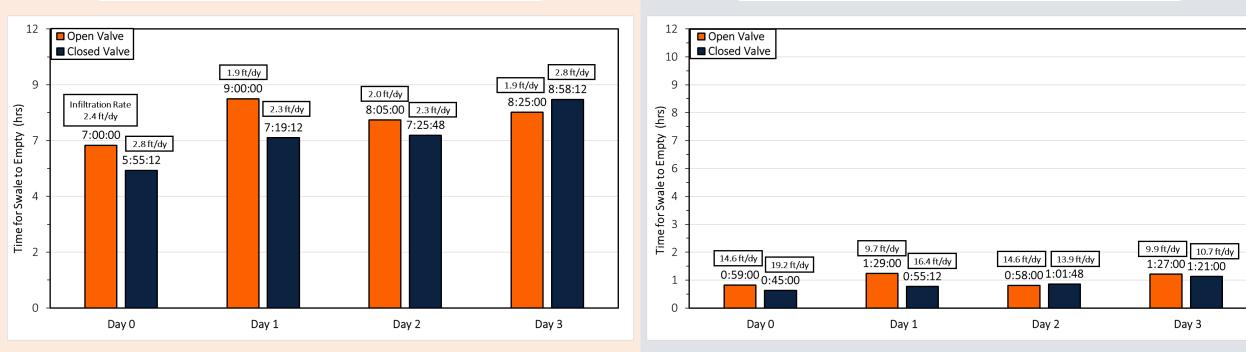
- Average drawdown time: 12 hours
 Closed Valve:
- Average infiltration rate: **2.5 ft/day**
- Average drawdown time: **7 hours**

- Average drawdown time: 5 hours
 Closed Valve:
- hrs . Average infiltration rate: 9.5 ft/day
 - Average drawdown time: 2.3 hours (
- **∆2.7** hrs

ALDOT SWALE

– JUNE –

MODIFIED SWALE



Closed average infiltration rate: **2.3 ft/day** Open average infiltration rate: **2.1 ft/day** Closed average infiltration rate: 15 ft/day

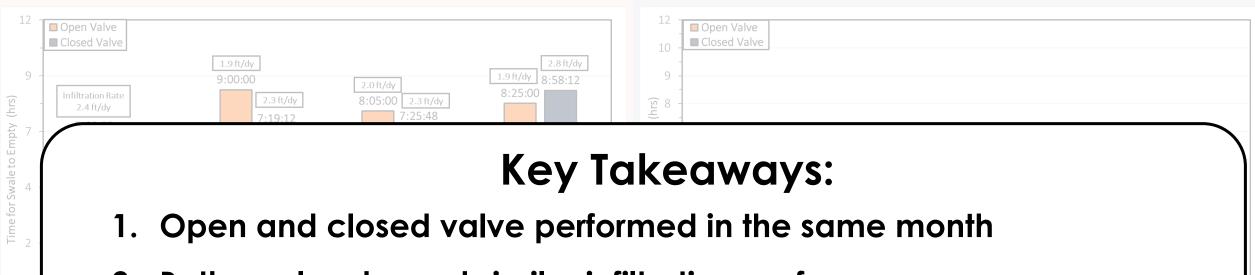
Open average infiltration rate: 12.3 ft/day

Difference between open and closed valve infiltration rate

averages is not statistically different

ALDOT SWALE

MODIFIED SWALE



- 2. Both swales showed similar infiltration performances
- 3. Seasonal variation appears to affect infiltration performance
- Open average infiltration rate: 2.1 ft/day

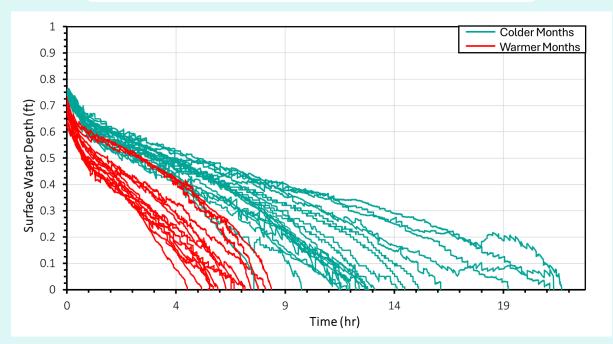
Open average infiltration rate: 12.3 ft/day

The difference between the open and closed valve infiltration rate averages is not big enough to be statistically significant.

SEASONAL VARIATION

ALDOT SWALE

MODIFIED SWALE

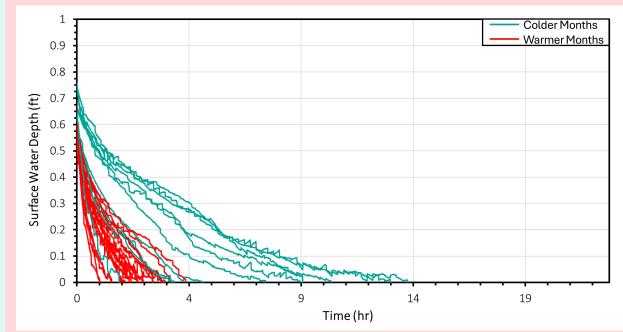


Colder Months:

- Average infiltration rate: 1.3 ft/day
- Average drawdown time: 14.4 hours

Warmer Months:

- Average infiltration rate: 2.2 ft/day
- Average drawdown time: 7.5 hours



Colder Months:

- Average infiltration rate: 2.7 ft/day
- Average drawdown time: **8.5 hours**
- Δ6.9 Warmer Months:

hrs

- Average infiltration rate: 7.2 ft/day
- Average drawdown time: 2.3 hours

∆6.2 hrs

SEASONAL VARIATION

MODIFIED SWALE ALDOT SWALE Warmer Month 0.7 (ft) 0.6 0.6 0.7 014 (ft) 0.6 **Key Takeaways:** 1. Colder months are associated with slower infiltration rates 2. Warmer months are associated with enhanced infiltration rates 3. Seasonal variation affects infiltration performance for both swales Co

hrs

- Average infiltration rate: 1.3 tt/day
- Average drawdown time: **14.4 hours** Warmer Months:
- Average infiltration rate: 2.2 ft/day
- Average drawdown time: 7.5 hours
- Δ6.9
 Average drawdown time: 8.5 hours
 Warmer Months:
 - Average infiltration rate: 7.2 ft/day
 - Average drawdown time: 2.3 hours +

Δ6.2

hrs

OVERALL PERFORMANCE

ALDOT Infiltration Swale





Avg. Infiltration Rate: 1.6 ft/day Avg. Drawdown: 12.25 hr

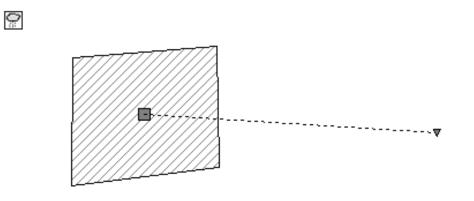


Avg. Infiltration Rate: 5.2 ft/day Avg. Drawdown: 5.06 hr

Modified Swale infiltration rate avg. is 3x greater than ALDOT Swale

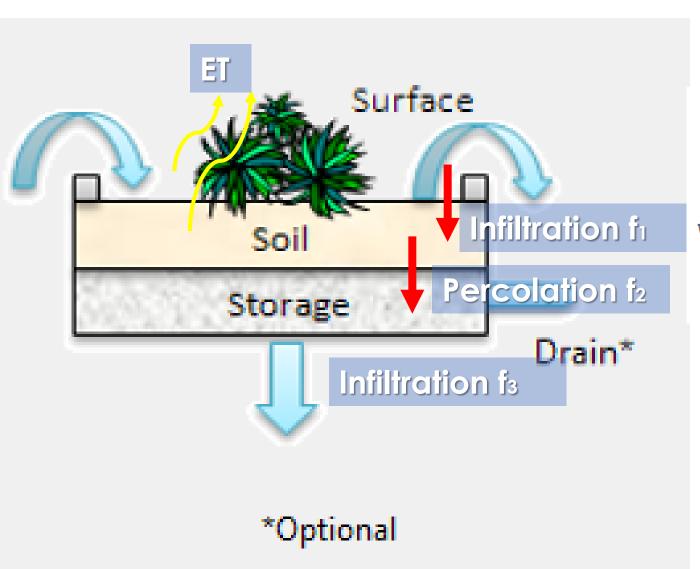
Modeling Infiltration Swale Performance w/ SWMM

- EPA Storm Water Management Model (SWMM)
- SWMM model development: a basin with the infiltration swale that fills the water (runoff) up to the maximum height of a ditch check (berm).



- Several model parameters (factors) are used to control the infiltration rate in each layer.
- Compare the observed and modeled drainage times for the SWMM model calibration.

SWMM Model basic parameters of Bio-retention (infiltration swale)



$$\phi_1 \frac{\partial d_1}{\partial t} = i + q_0 - e_1 - f_1 - q_1 \qquad \text{Surface Layer} \qquad (6-1)$$

$$D_2 \frac{\partial \theta_2}{\partial t} = f_1 - e_2 - f_2 \qquad \text{Soil Layer} \qquad (6-2)$$

(6-3)

 $\phi_3 \frac{\partial d_3}{\partial t} = f_2 - e_3 - f_3 - q_3$ Storage Layer

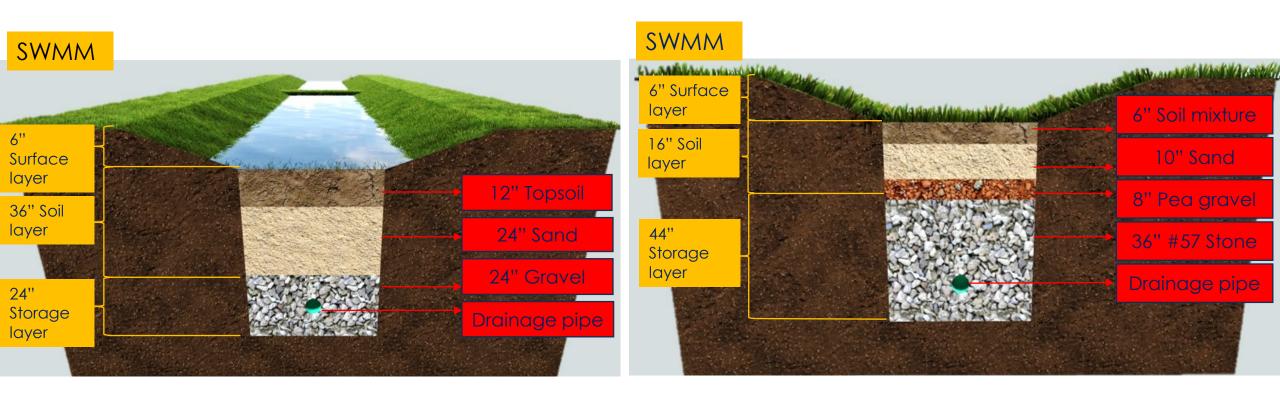
where:

- d1 = depth of water stored on the surface (ft),
- θ_2 = soil layer moisture content (volume of water / total volume of soil),
- d₃ = depth of water in the storage layer (ft),

$$f_1 = K_{2S} \left(1 + \frac{(\phi_2 - \theta_{20})(d_1 + \psi_2)}{F} \right)$$

 $f_{2} = \begin{cases} K_{2S} exp(-HCO(\phi_{2} - \theta_{2})), & \theta_{2} > \theta_{FC} \\ 0, & \theta_{2} \le \theta_{FC} \end{cases}$

Constructing & Evaluating Infiltration Swales



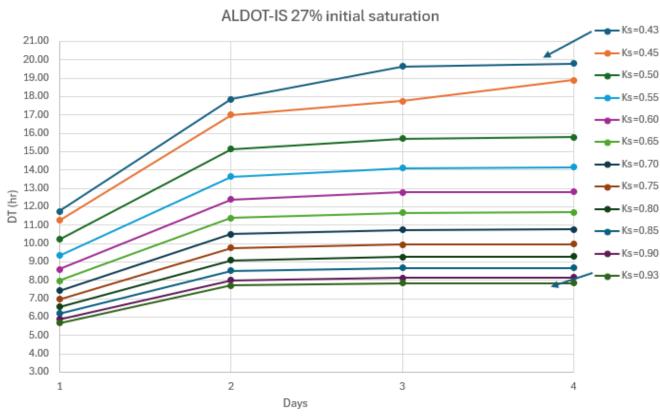
ALDOT Swale

AU Swale

Drainage Time (TD) versus Soil Conductivity For One-day Dry Period

ALDOT-IS

% Initial saturated = 27 % Seepage rate = 0.43 in/hr

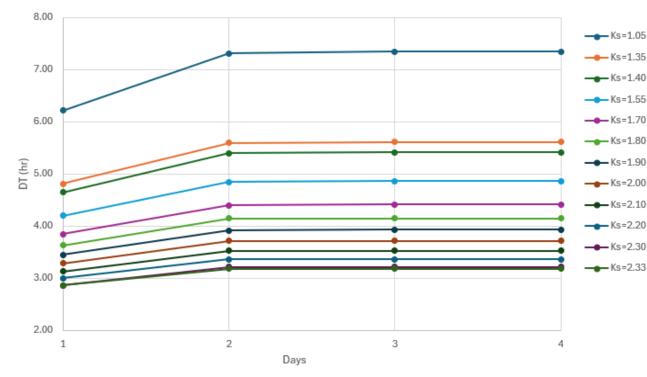


Time	Observed (TD)	Ks of Soil	Simulated (TD)
Jan-Feb	12.70 ± 0.30	0.55	12.81 ± 2.02
April	6.56 ± 0.90	1.05	6.39 ± 0.77
June (open)	8.45 ± 0.75	0.80	8.55 ± 1.15
June (close)	7.76 ± 1.16	0.90	7.54 ± 0.97

Drainage Time (TD) versus Soil Conductivity For One-day Dry Period

AU-EIS % Initial saturated = 54 % Seepage rate = 0.43 in/hr

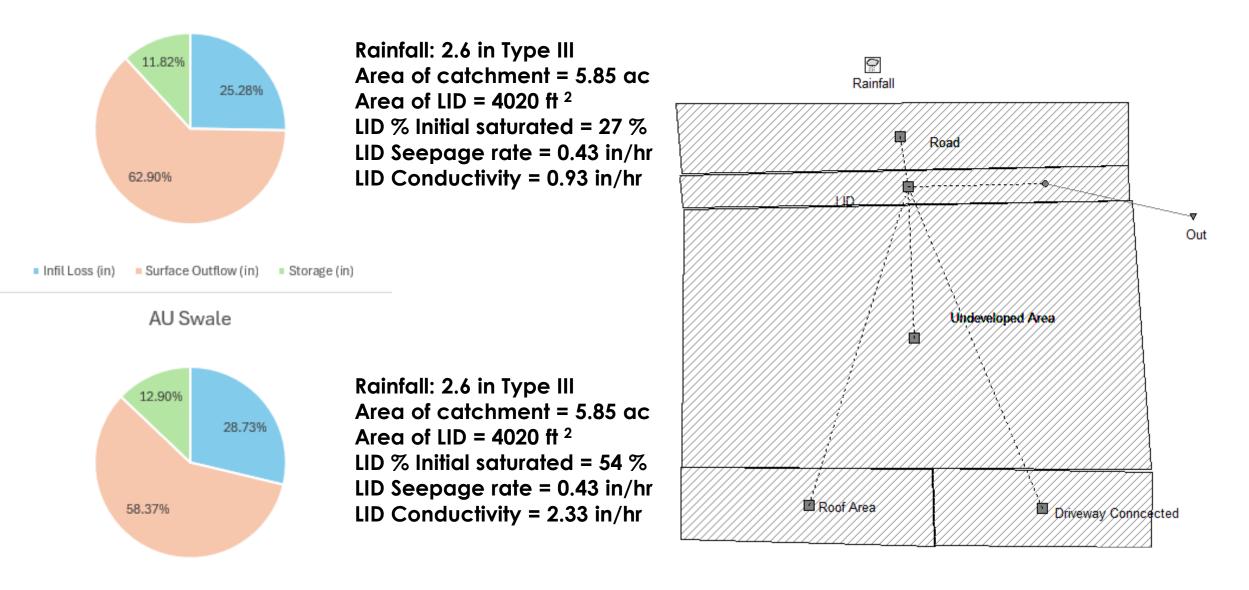
AU-EIS 54% initial saturation



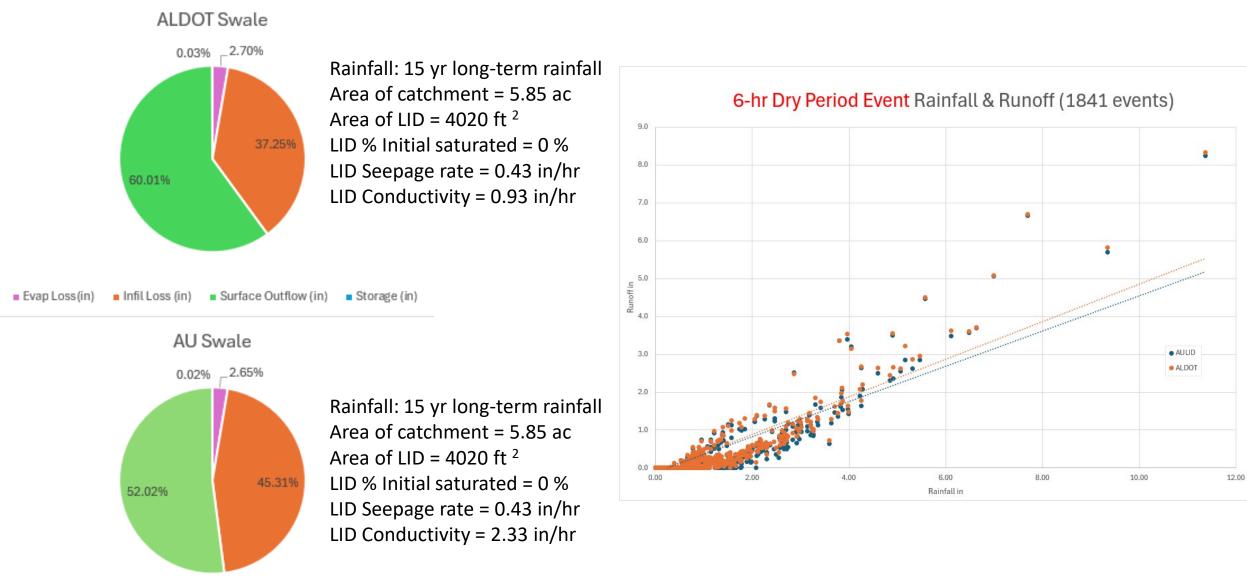
1.05				
1.35				
1.40	Time	Observed	Ks of	Simulated
1.55		(TD)	Soil	(TD)
=1.70		נטו)	3011	(ישו)
=1.80 =1.90	Jan-Feb	8.55 ± 3.65	0.85	8.75 ± 0.69
2.00	April	3.63 ± 0.78	2.00	3.61 ± 0.19
=2.20	June (open)	1.23 ± 0.27	5.4	1.32 ± 0.03
=2.30 =2.33	June (close)	1.03 ± 0.22	6.2	1.15 ± 0.02

Swale's Runoff-control Performance at Design Rainfall

ALDOT Swale



Long-term (Continuous) SWMM Modeling



SWMM Modeling Conclusions

- In the field-scale test, the average drainage time for ALDOT-IS ranged from 7.8 to 12.7 hours and from 1.03 to 8.6 hours for AU-EIS for a one-day dry period.
- The infiltration swale was modeled using SWMM, and the average drainage time for AU-EIS and ALDOT-IS does not change hours when the native soil's saturated hydraulic conductivity increases from 0.3 in/hr to 1.2 in/hr (Hydrological Soil Group B).
- Under 24-hour design rainfall (95th percentile rainfall), AU-EIS has 4.54% less runoff, 3.45% more infiltration, and 1.09% more water in storage.
- Under long-term simulation (15 years), AU-EIS has 8.00% less runoff, 8.07% more infiltration.





DOES THIN LAYER PLACEMENT LEAD TO REPLANTING LIVING SHORELINES?

MITIGATION METHODS

GREEN – SOFTER TECHNIQUES

GREY – HARDER TECHNIQUES

Living Shorelines



VEGETATION

Provides a buffer to upland areas and breaks small waves. Suitable for low wave energy environments.

EDGING Added structure holds the toe of existing or vegetated slope in place. Suitable for most areas except high wave energy environments.



SILLS Parallel to vegetated shoreline, reduces wave energy, and prevents erosion. Suitable for most areas except high wave energy environments.

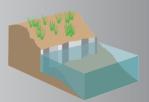


BREAKWATER (vegetation optional) Offshore structures intended to break waves, reducing the force of wave action, and encourage sediment accretion. Suitable for most areas.

Coastal Structures



REVETMENT Lays over the slope of the shoreline and protects it from erosion and waves. Suitable for sites with existing hardened shoreline structures.



BULKHEAD Vertical wall parallel to the shoreline. Intended to hold soil in place. Suitable for high energy settings and sites with existing hard shoreline structures.



GRADIENT OF WAVE ENERGY



Photo credit: CLIMAtlantic

LIVING SHORELINES TYPES

Non-Structural

Tidal Marsh

Fiber Logs

Beach Nourishment

Riparian Forest

Bank Grading

Hybrid

Marsh Sill

Shellfish Reef Community

Shellfish Reef

Pre-Cast Reef Structures

Bagged Shells

LIVING SHORELINES – MARSHES

Marsh Benefits:

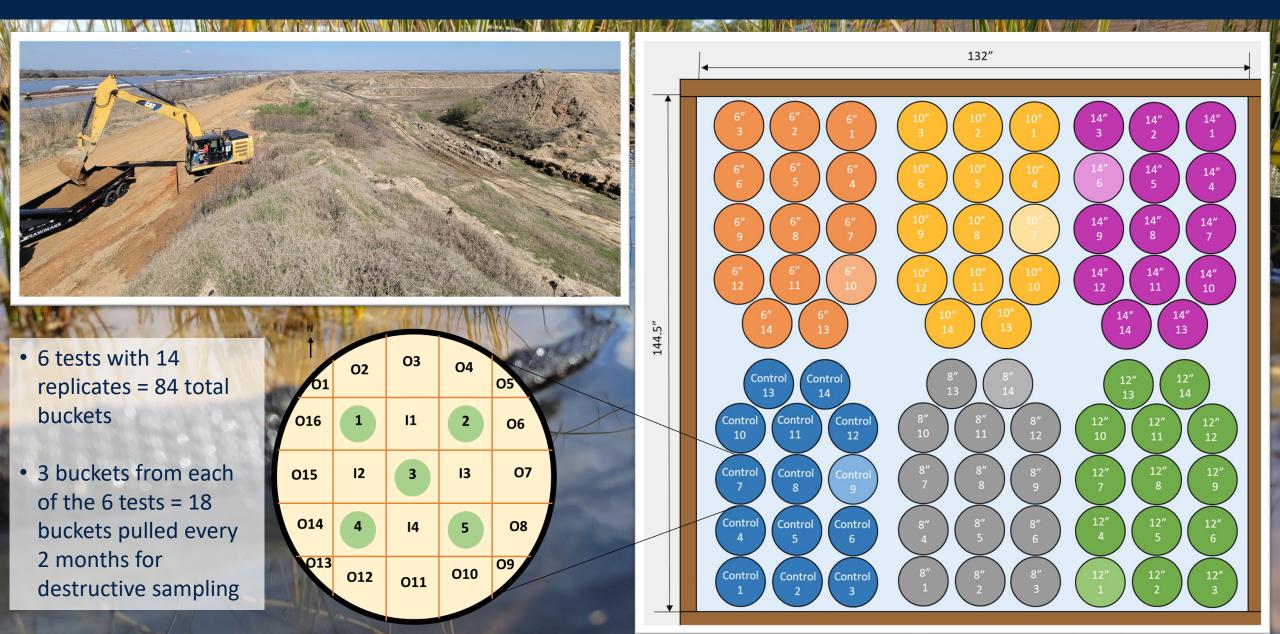
Habitat Biodiversity Pollution control Flood protection Carbon storage Wave damping Buffering Stormwater storage Sediment capture & retention

% Wave Energy to Marsh Length

- 50% dissipated in the first 8 ft
 - 100% dissipated in 100 ft

Sea Level Rise has consumed 25–50% of salt marshes globally

EXPERIMENTAL DESIGN



EXPERIMENTAL DESIGN

Phase I

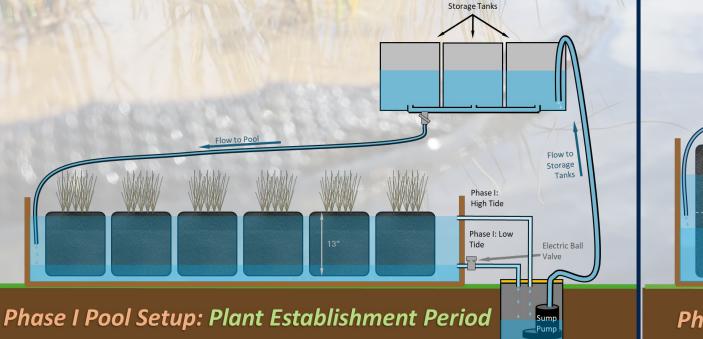
- Phase I gave plants 4 months to establish before proceeding to Phase II
- Tides were set to 3 in. Low Tide and 13 in. High Tide

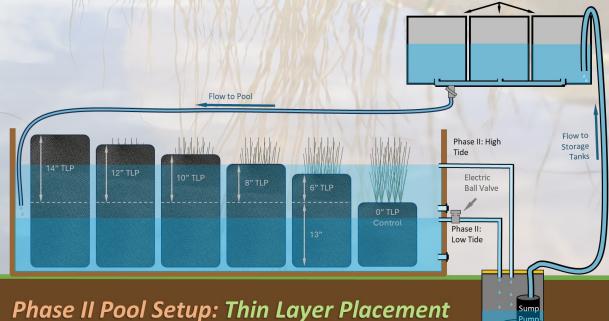
Phase II

- Simulated sea level rise and increased tidal heights to 11 in. Low Tide and 21 in. High Tide
- Phase II will last 5 months before concluding the experiment



Storage Tanks





EXPERIMENTAL DESIGN



DATA COLLECTION - MONTHLY LIVE DATA COLLECTION

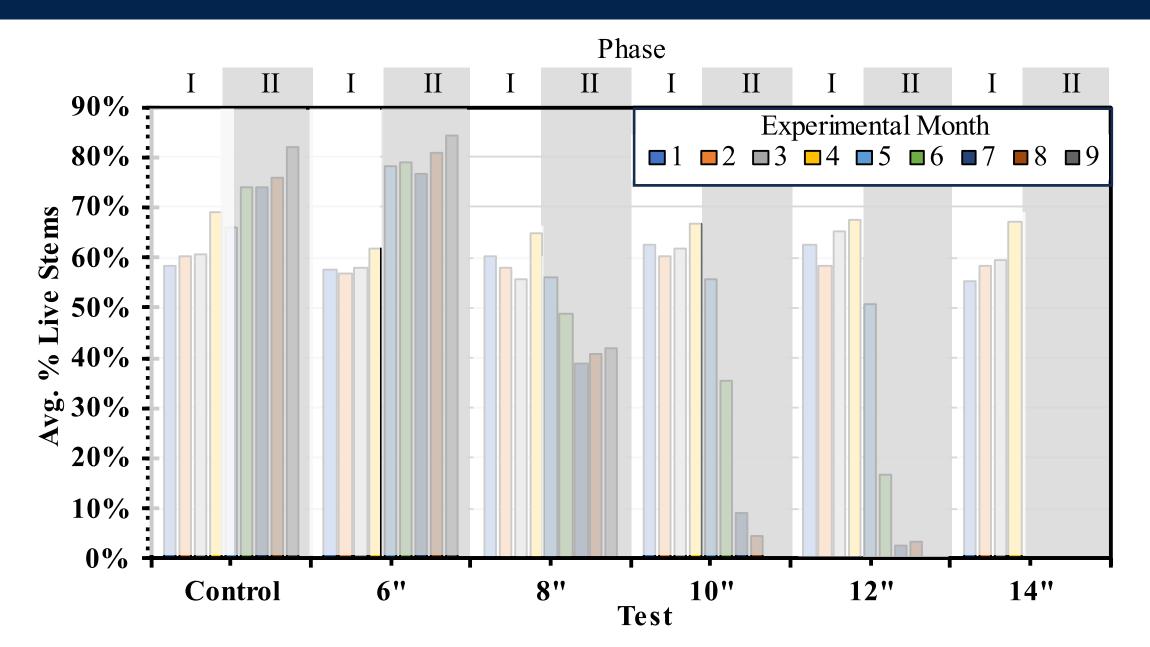


Data collected: Total shoot count (live:dead/grid), shoot length (average of 30 random/bucket), shoot basal diameter (average of 30 random/bucket), and soil compaction (height of soil in bucket from floor)

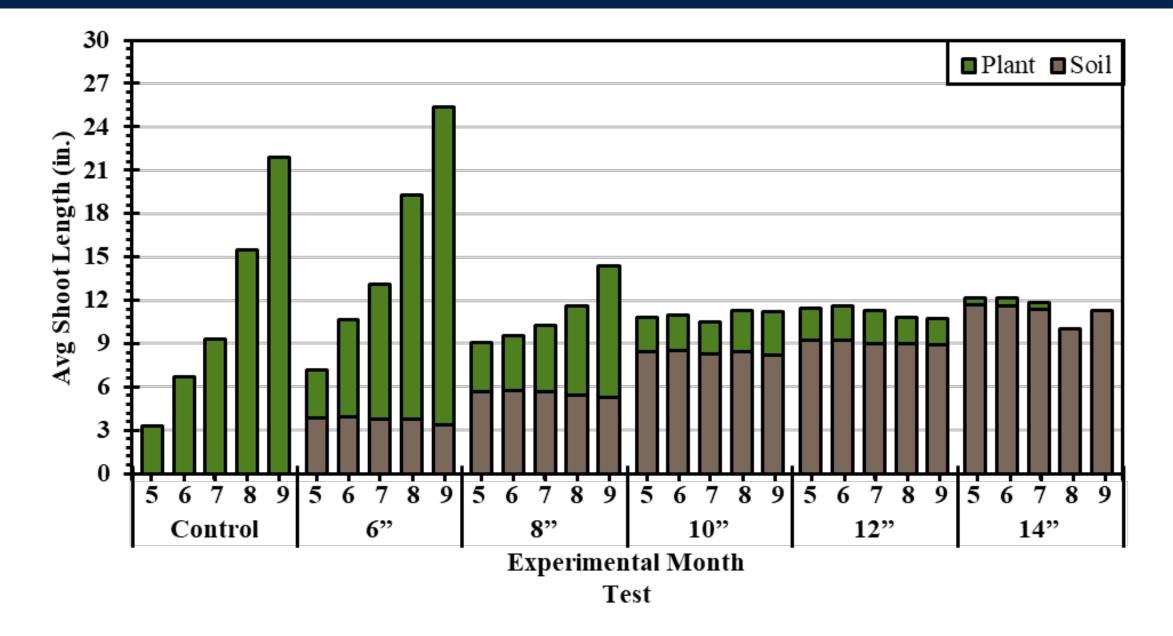
DATA COLLECTION - BI-MONTHLY DESTRUCTIVE SAMPLING



SHOOT SURVIVAL



PHASE II SHOOT LENGTH



PHASE II PLANT RECOVERY



RESULTS & IMPACT

- 6 in. TLP application depth (47% buried) demonstrated consistent growth and adaptation
- Threshold exists between 8-10 in. TLP application depth (62-78% buried)
- ≥10 in. TLP application depth (≥78% buried) results in significant stress with no recovery

- Under moderate TLP, replanting would not be necessary
- Future Study: Can the plants survive w/out thin layer placement

QUESTIONS?



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